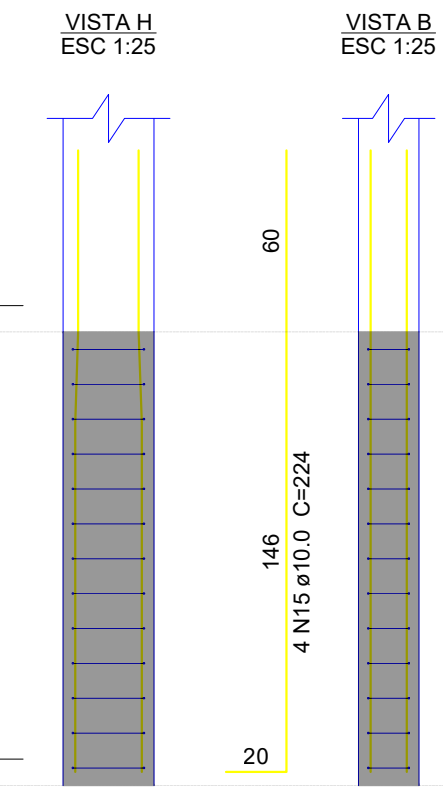
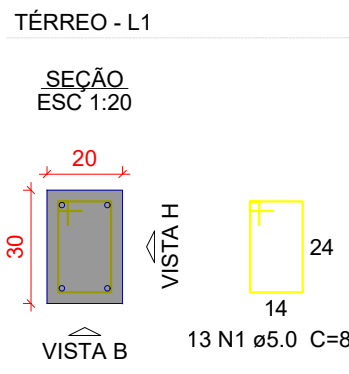
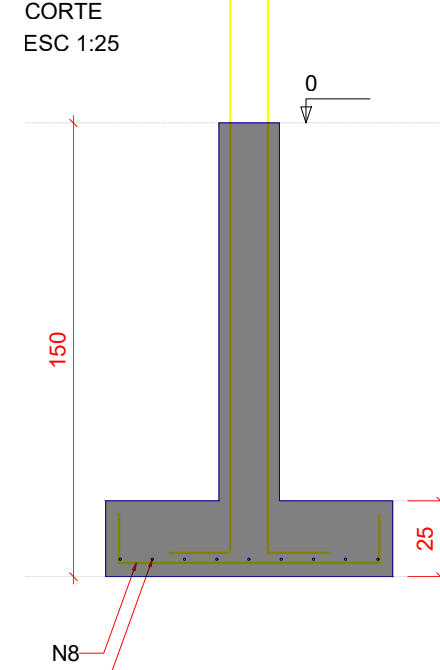
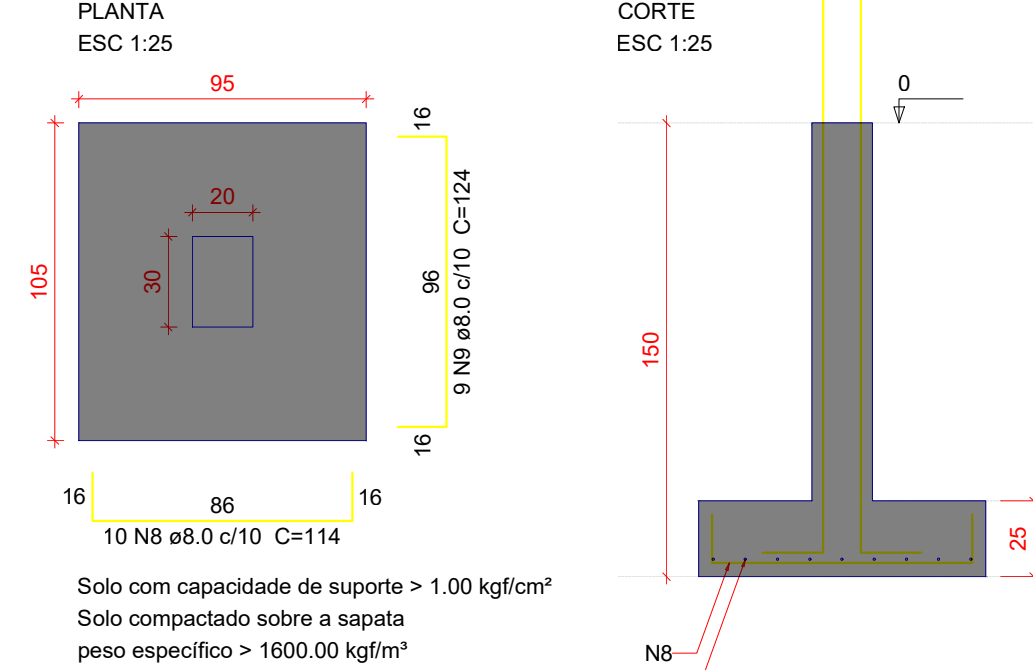
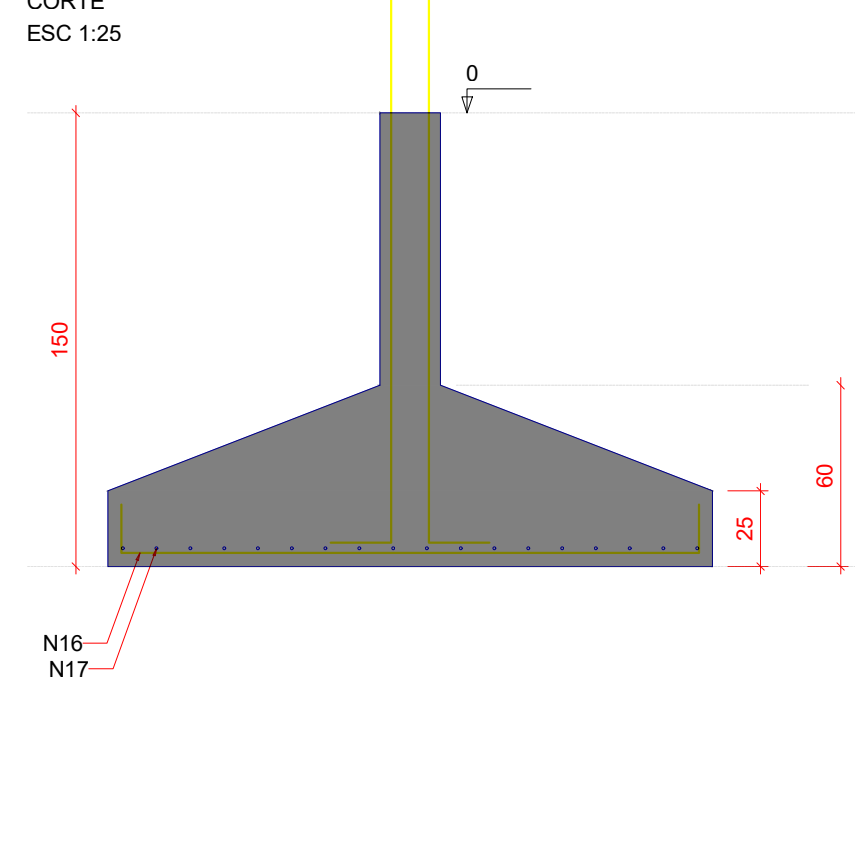
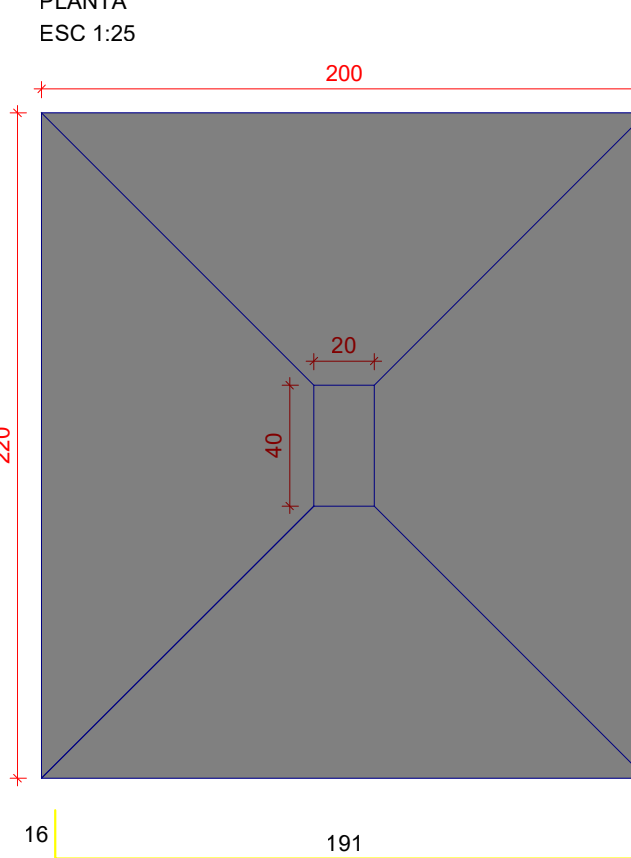


P132=P161=P162

S132=S161=S162



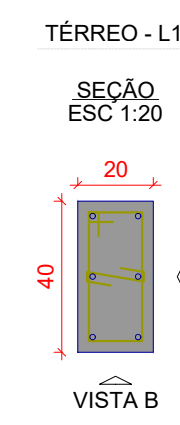
S146



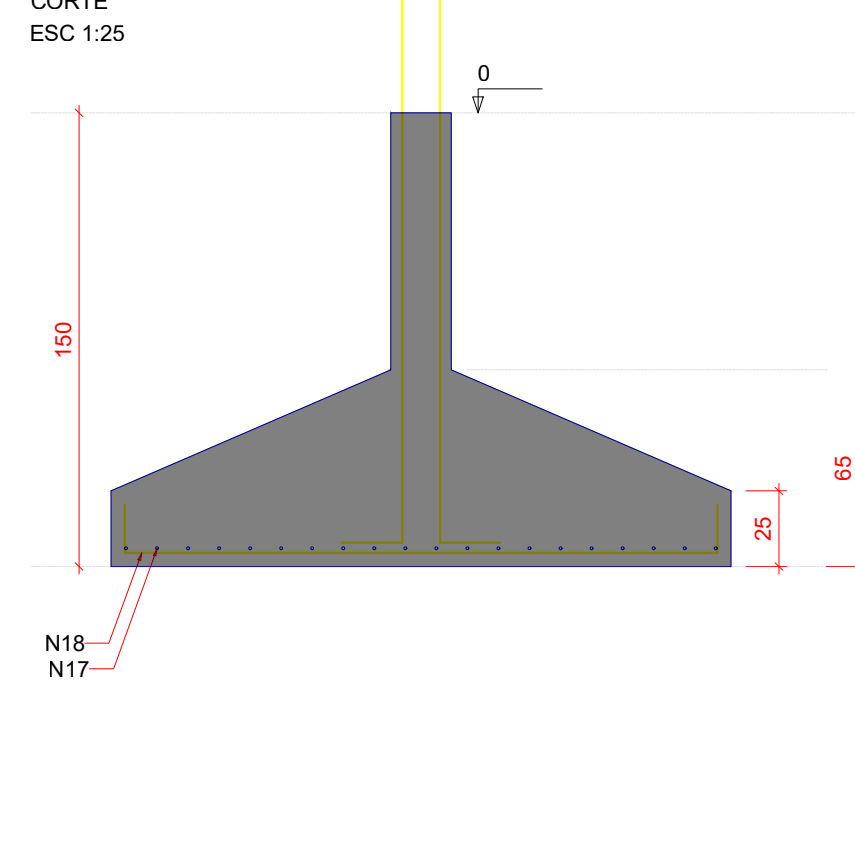
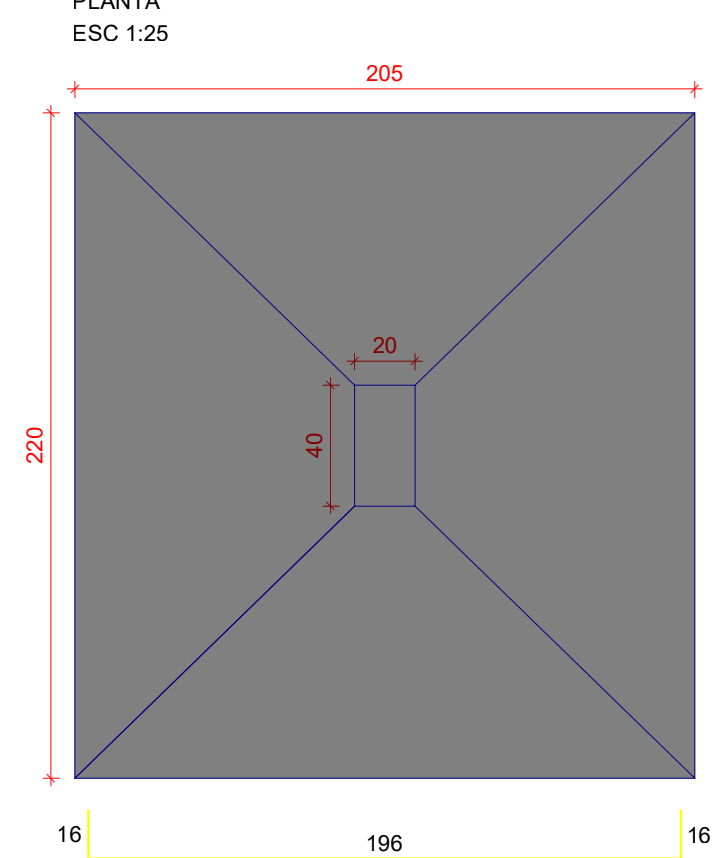
P146

VISTA H  
ESC 1:25

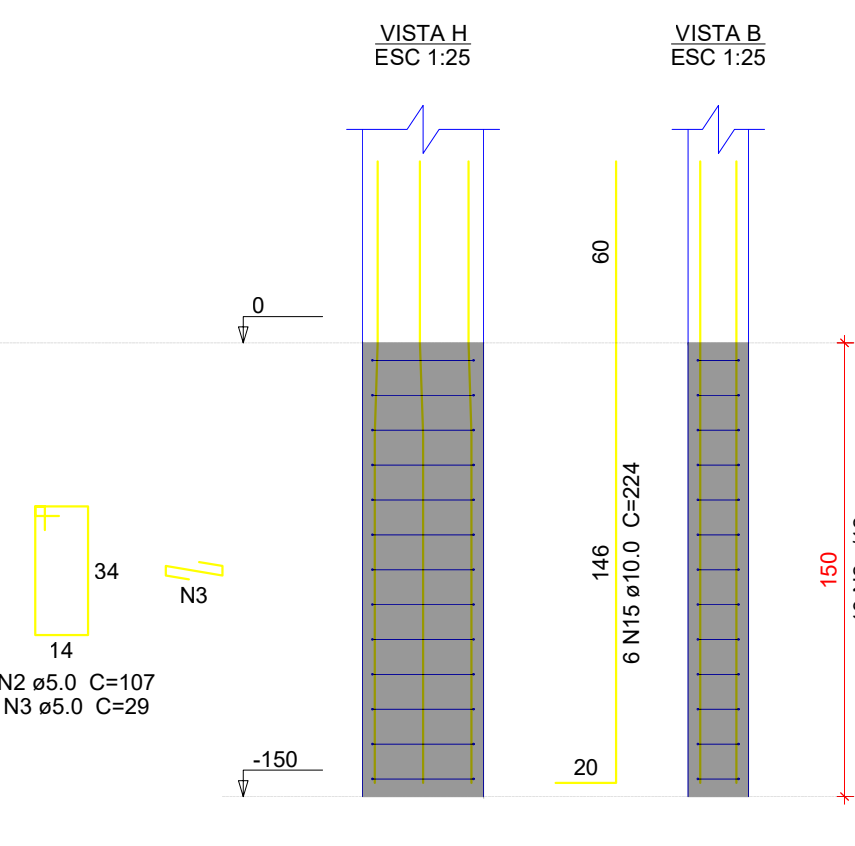
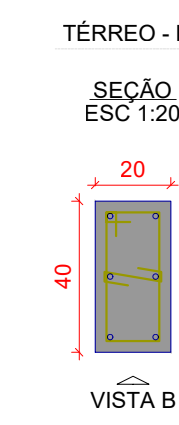
VISTA B  
ESC 1:25



S147



P147



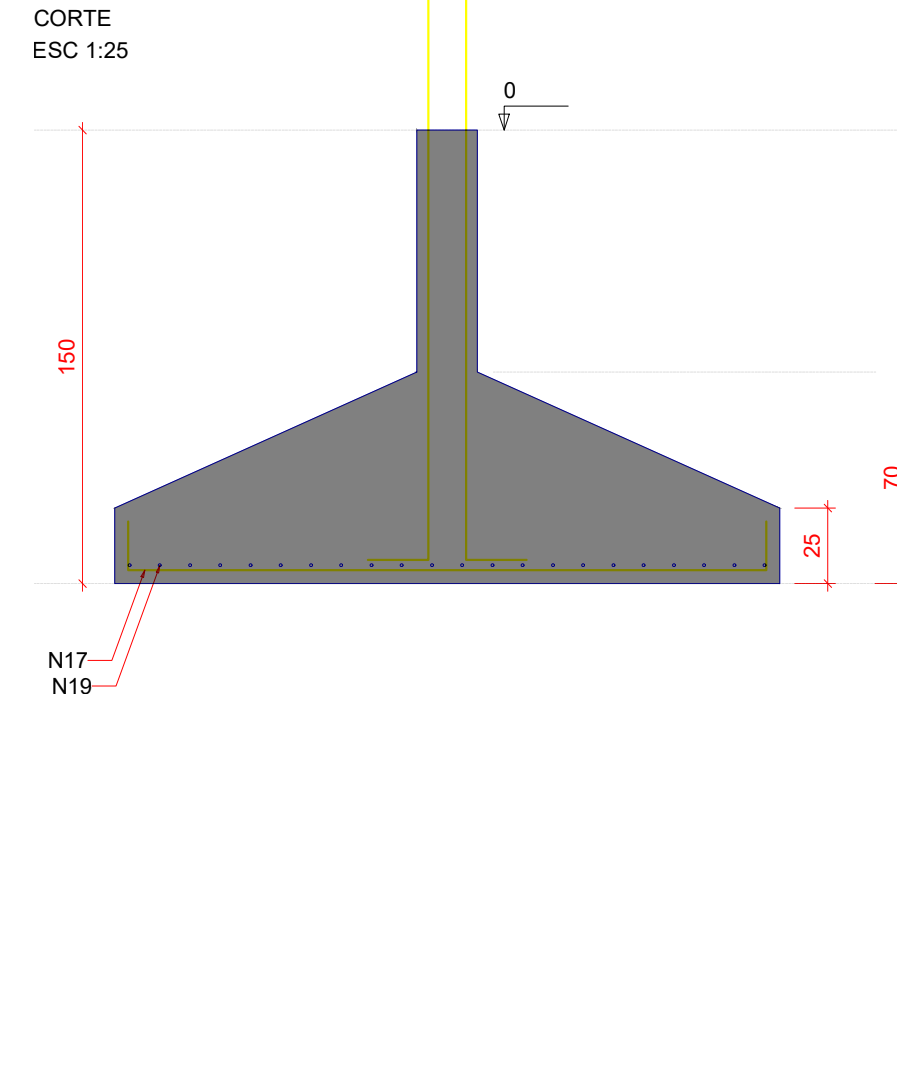
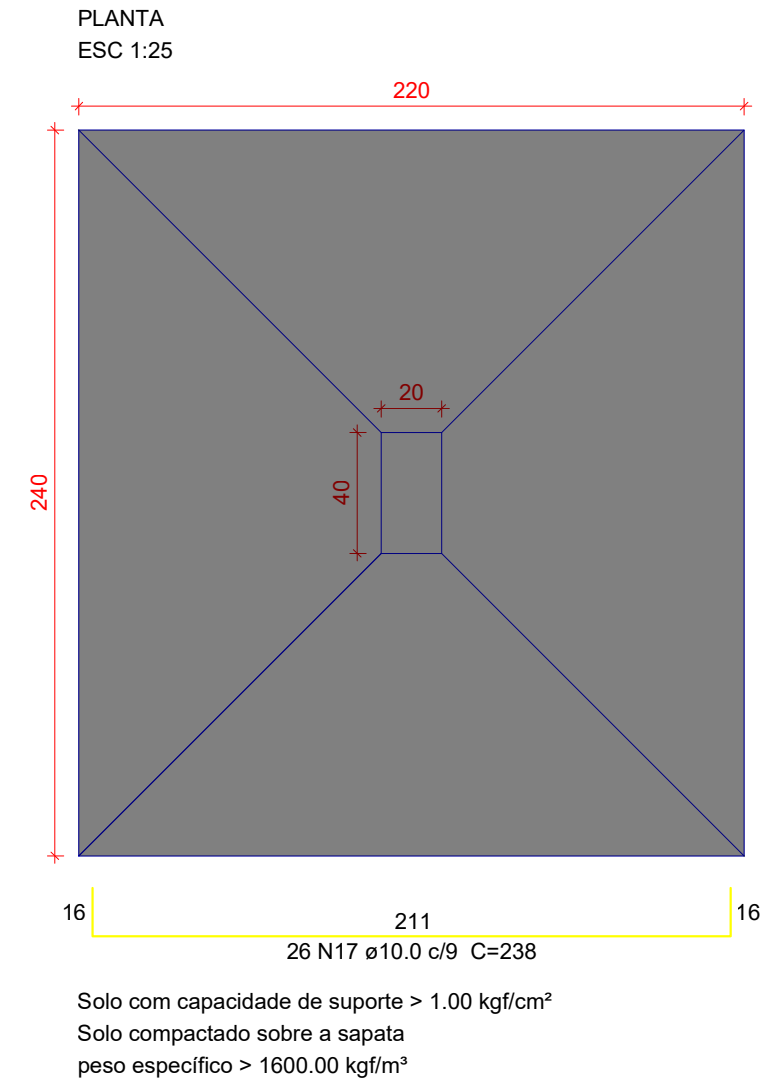
Relação do aço

CA50	N	DIAM (mm)	QUANT	CUNT (cm)	C.TOTAL (cm)
1	5.0	52	67	4524	
2	5.0	52	107	5564	
3	5.0	52	29	1508	
4	5.0	39	97	3763	
5	5.0	65	24	1560	
6	5.0	13	107	1391	
7	5.0	13	117	1521	
8	8.0	30	114	3420	
9	8.0	27	124	3346	
10	8.0	21	209	4389	
11	8.0	21	154	3234	
12	8.0	14	189	2646	
13	8.0	15	134	2010	
14	8.0	11	159	1749	
15	10.0	70	224	15680	
16	10.0	22	218	4706	
17	10.0	64	238	12532	
18	10.0	36	223	8528	
19	10.0	22	258	5879	
20	10.0	42	203	8526	
21	10.0	32	228	7296	
22	10.0	18	193	3474	
23	10.0	16	198	3006	
24	10.0	25	293	8204	
25	12.5	20	282	5640	

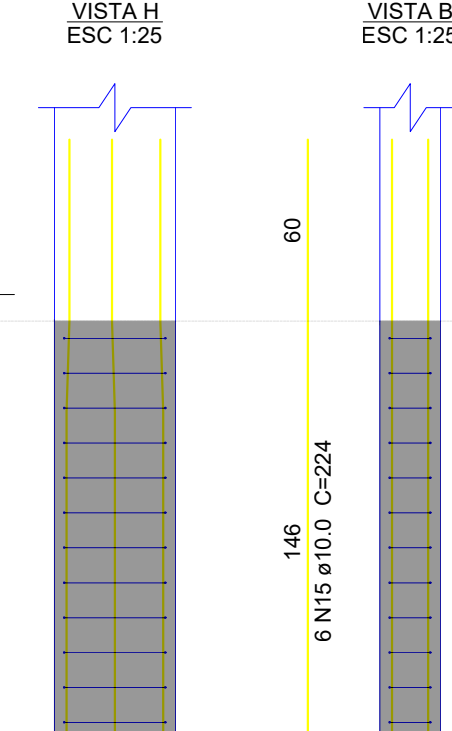
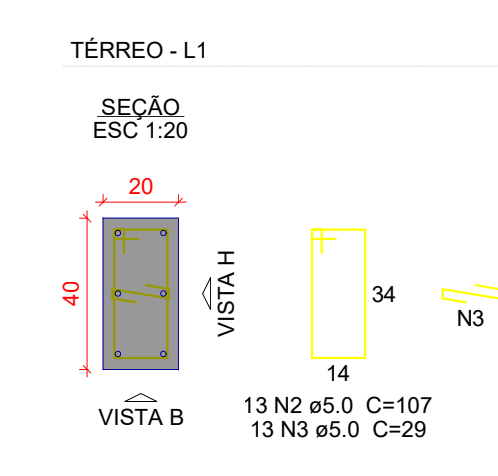
Resumo do aço

AÇO	DIAM (mm)	C.TOTAL (m)	PESO + 10 % (kg)
CA50	8.0	206	90.3
	10.0	799.2	542
	12.5	56.4	59.8
CA60	5.0	198.6	33.7
PESO TOTAL (kg)			
CA50	692		
CA60	33.7		
Volume de concreto (C-35) = 17.22 m³			

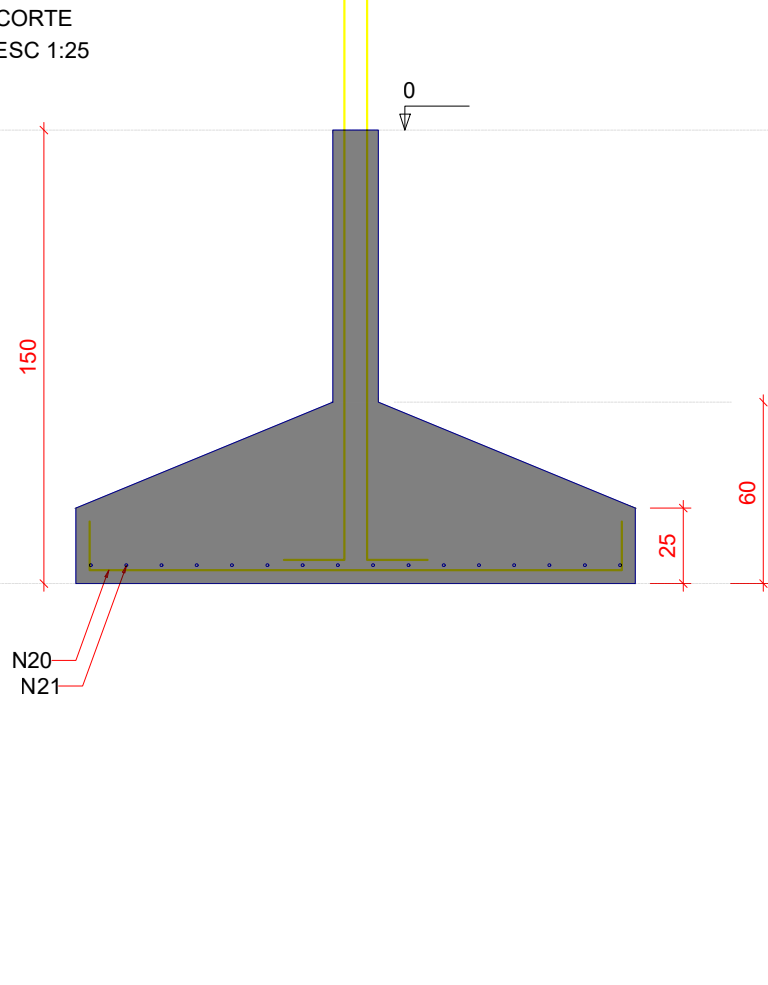
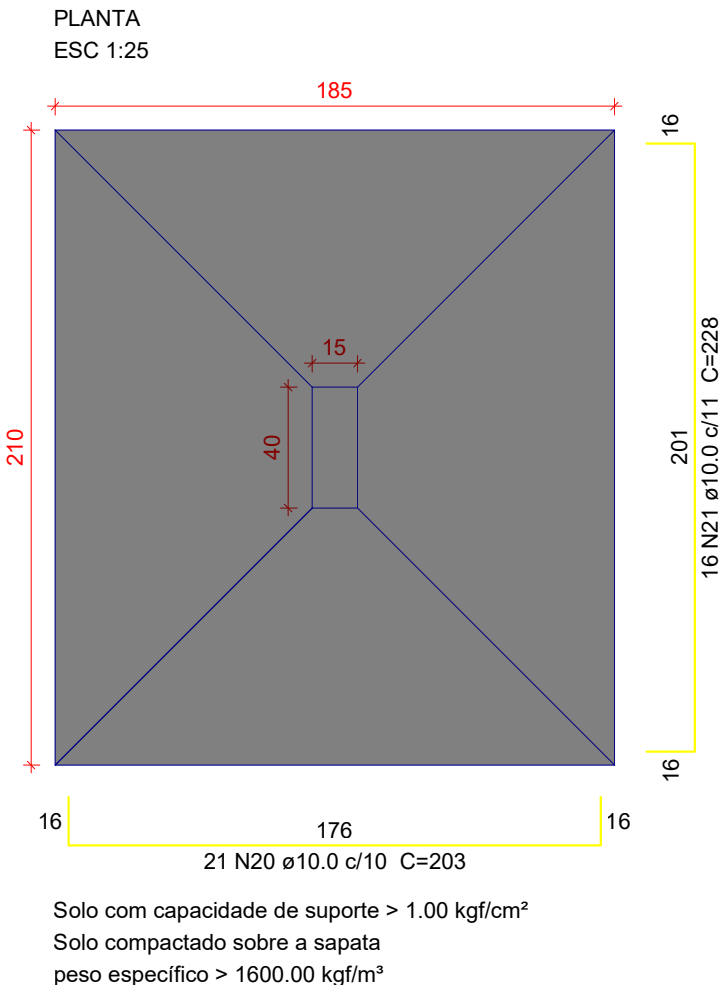
S148



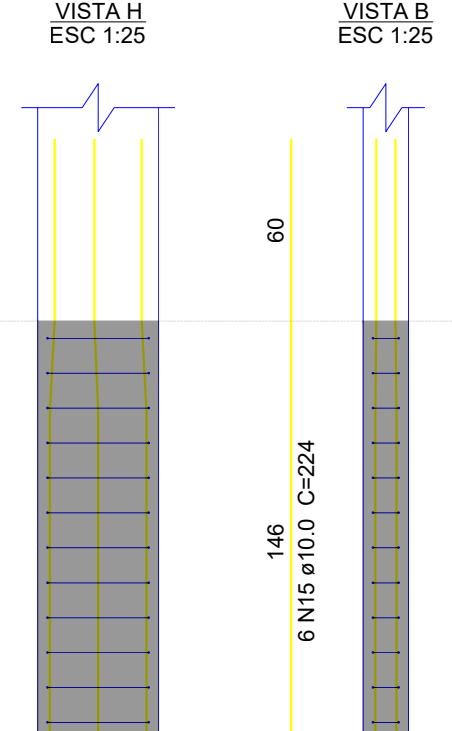
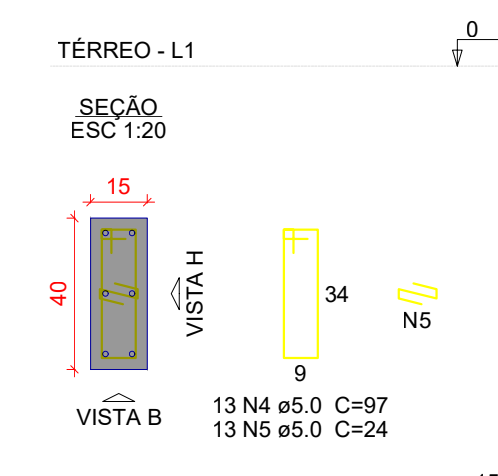
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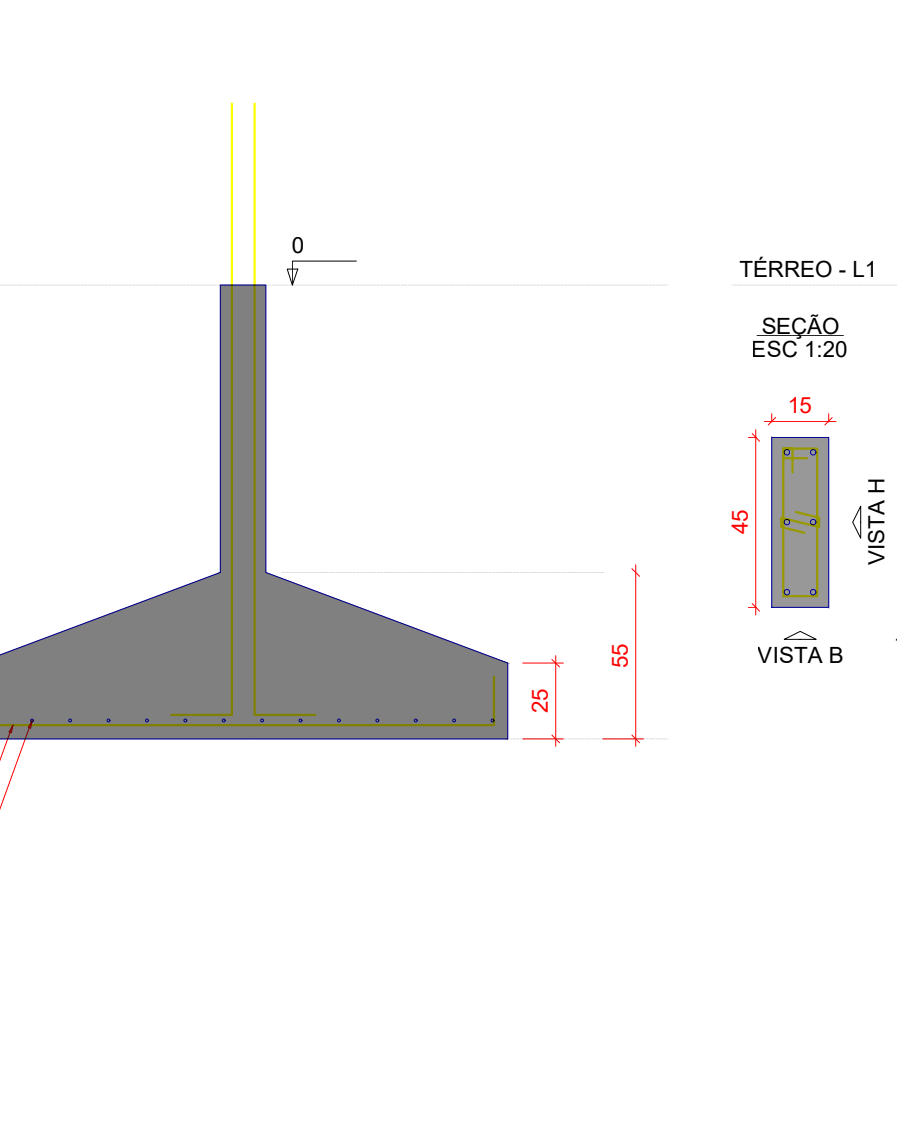
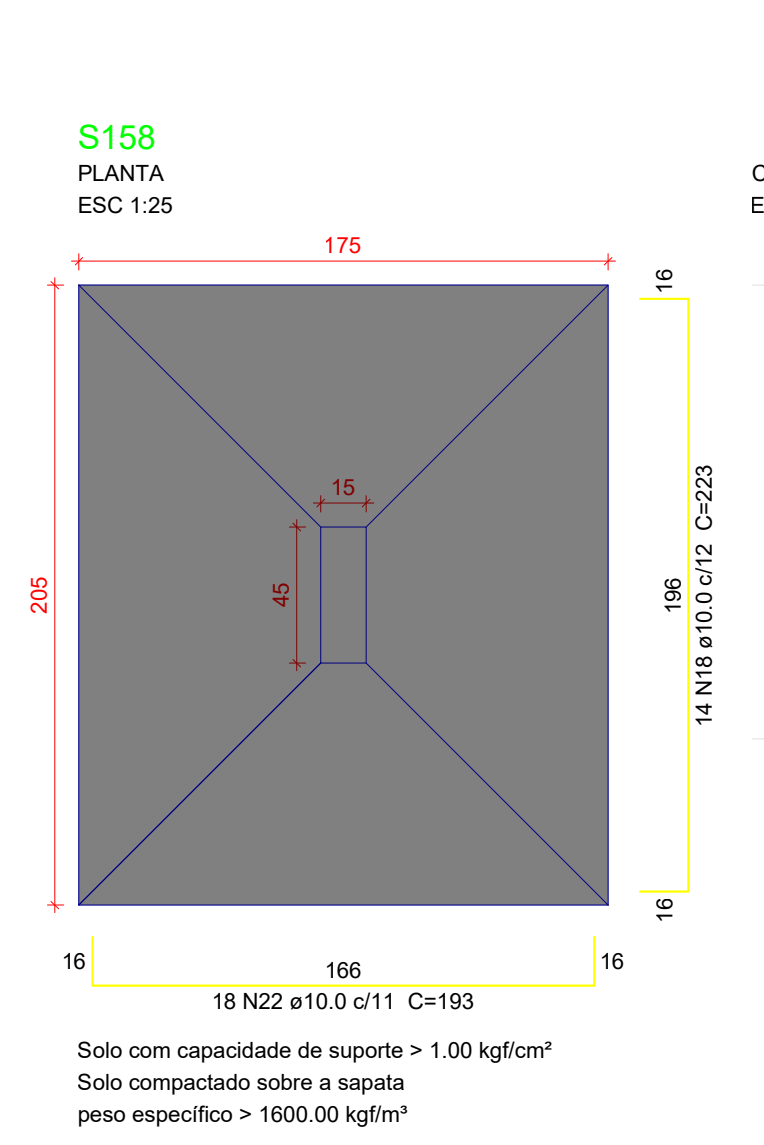
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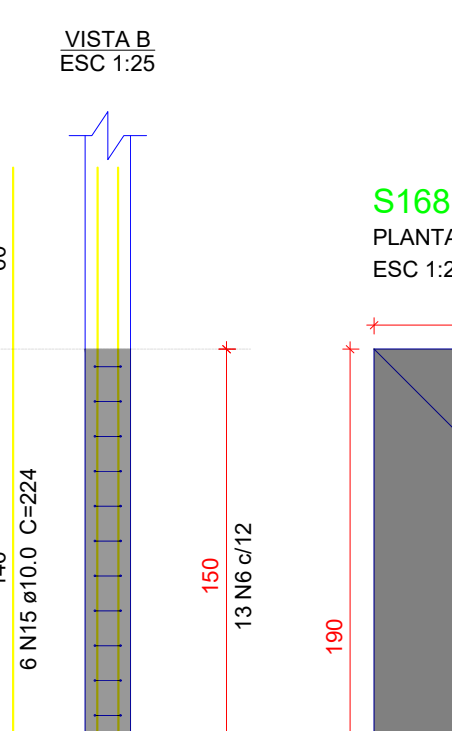
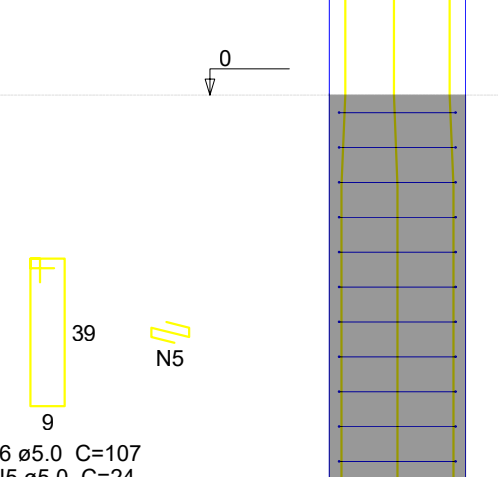
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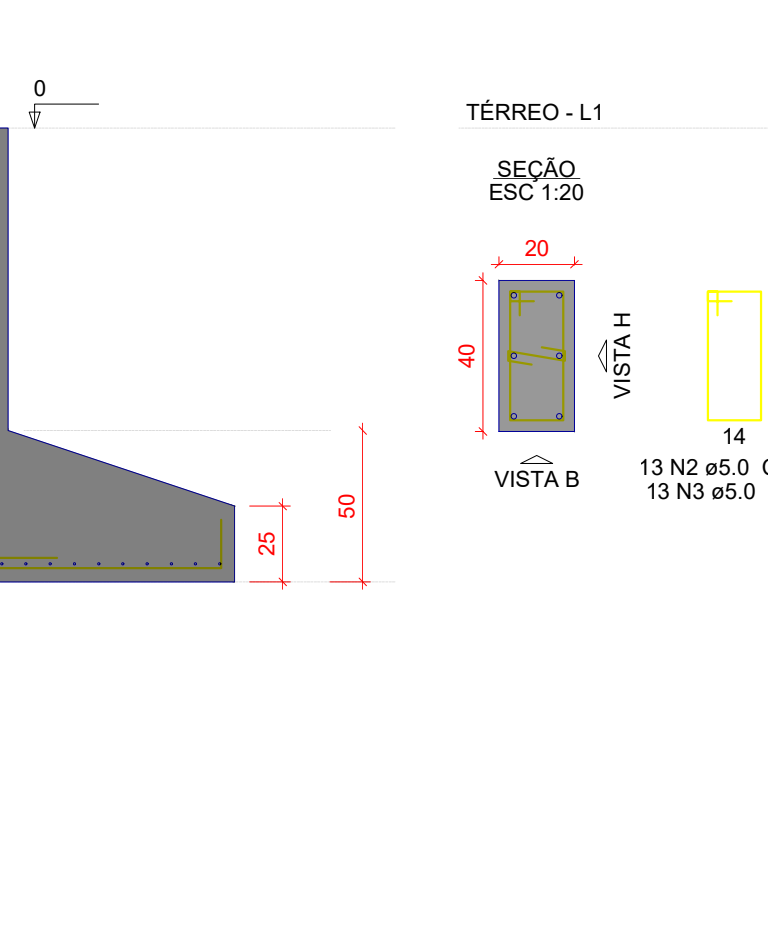
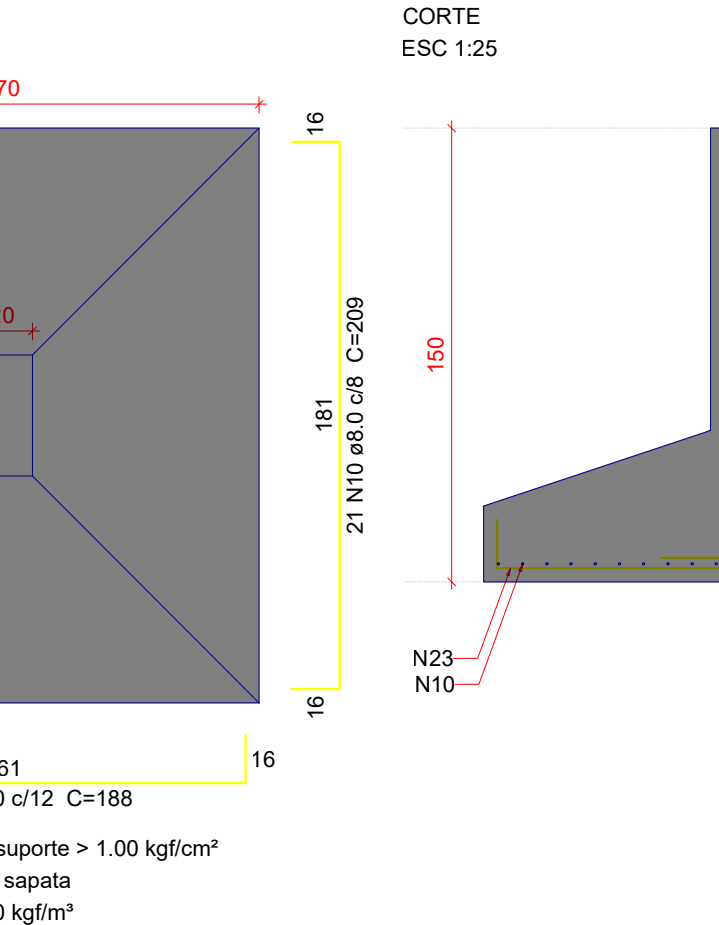
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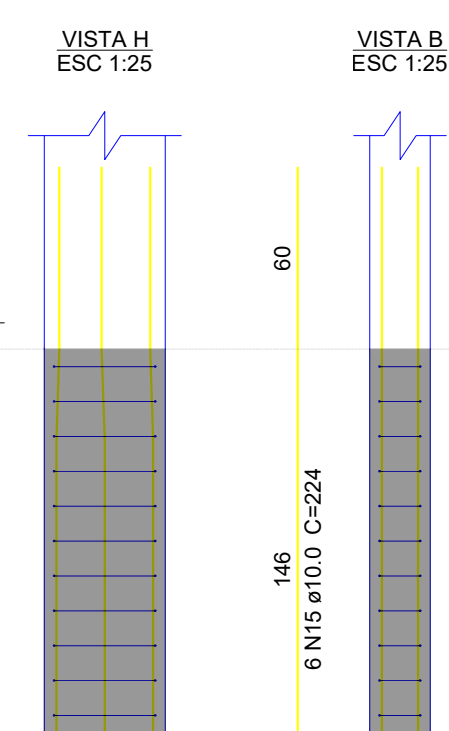
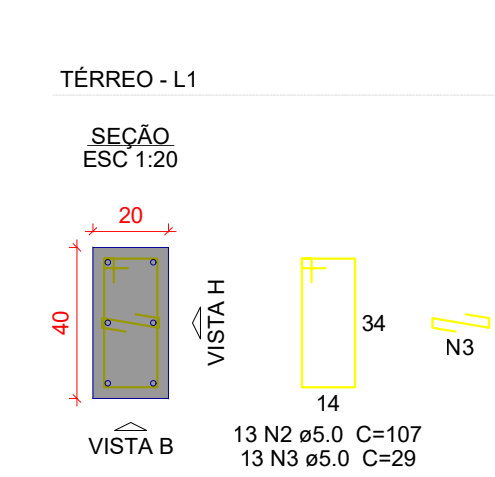
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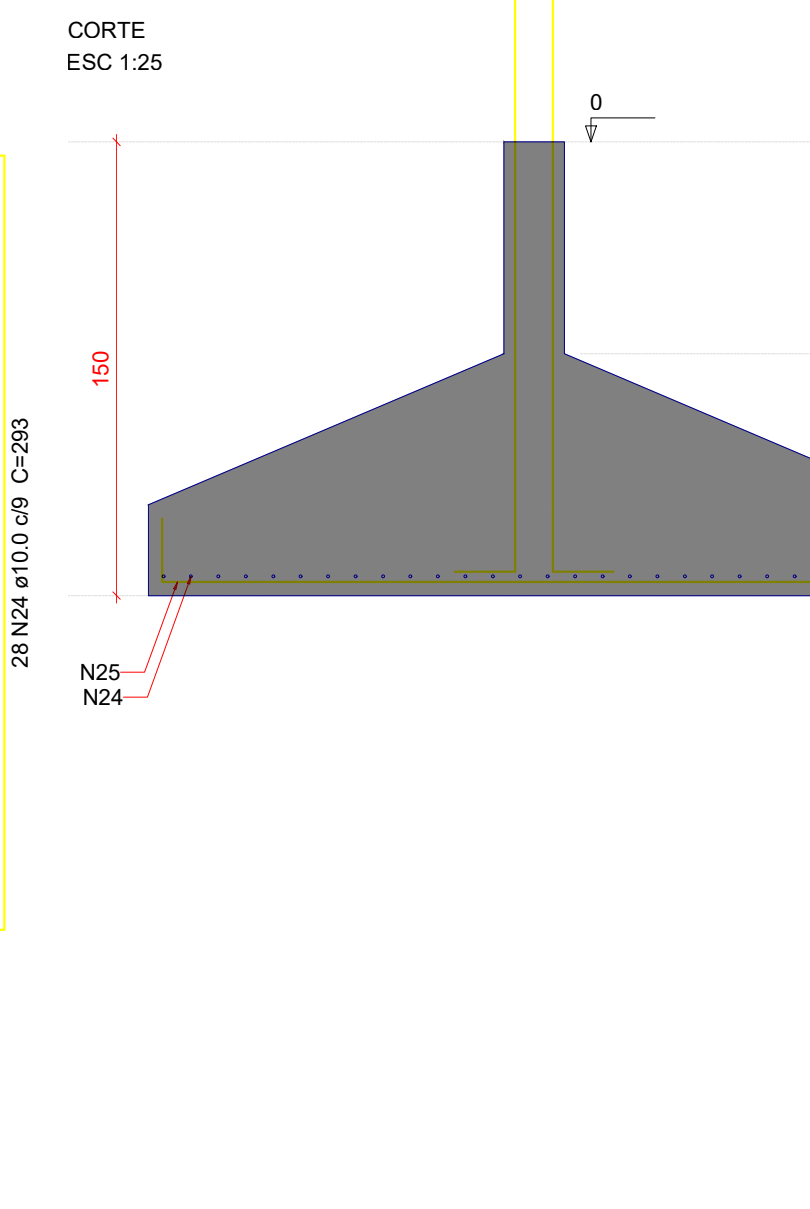
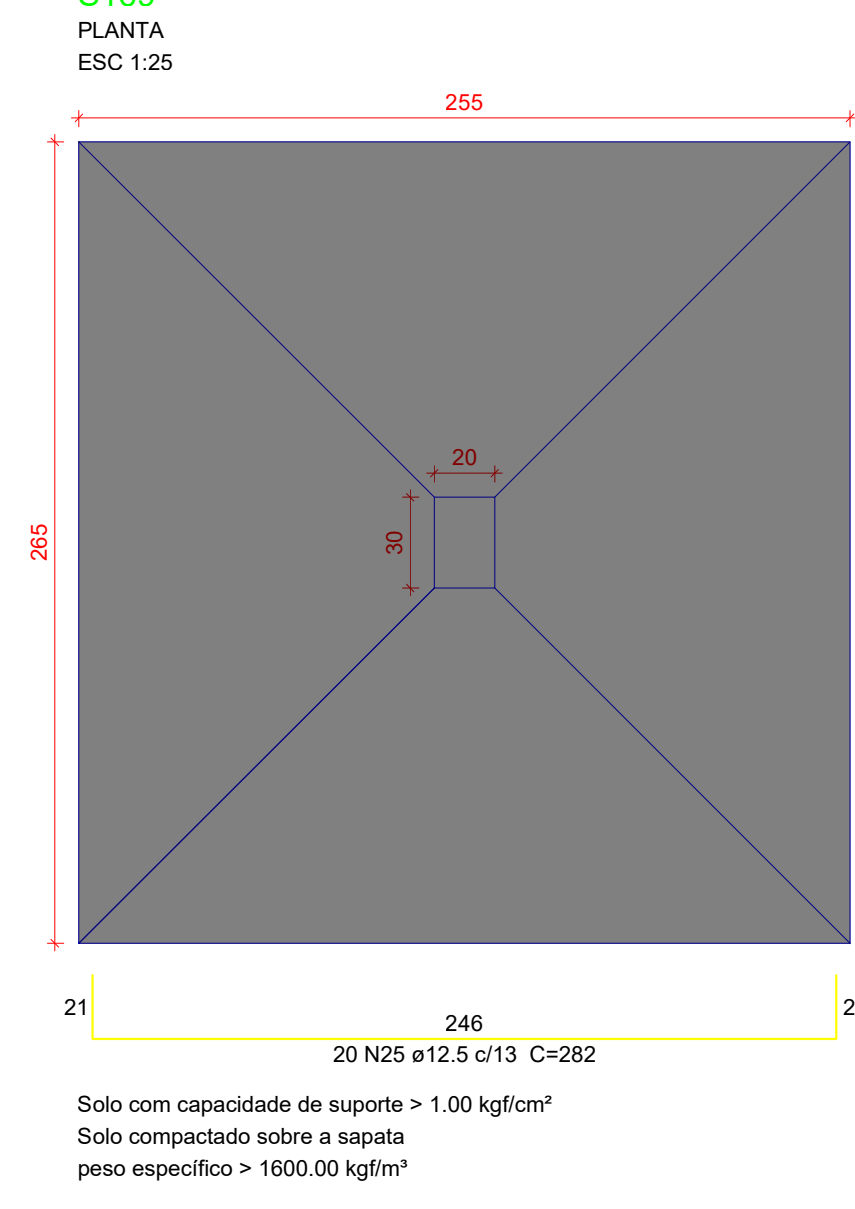
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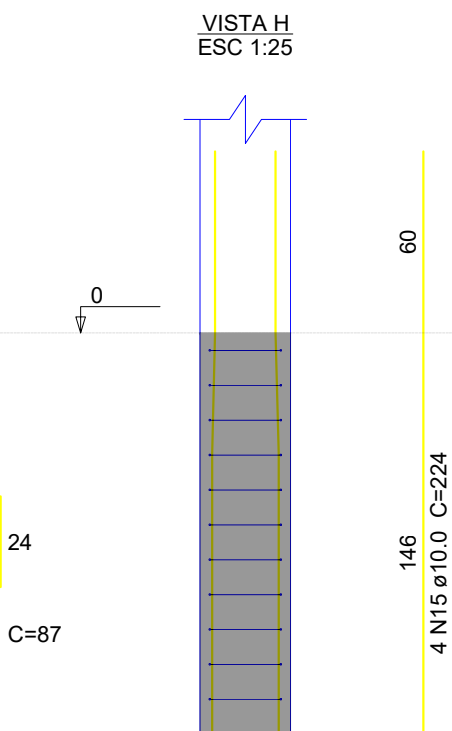
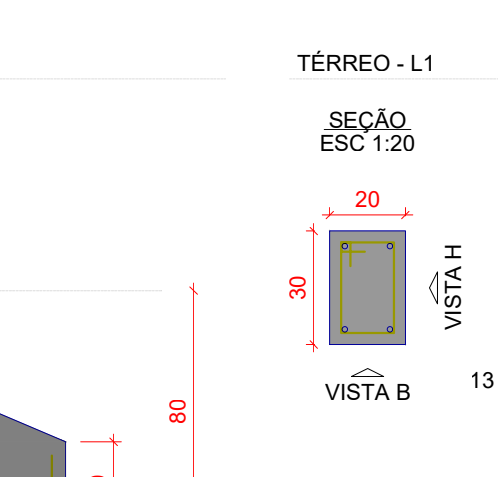
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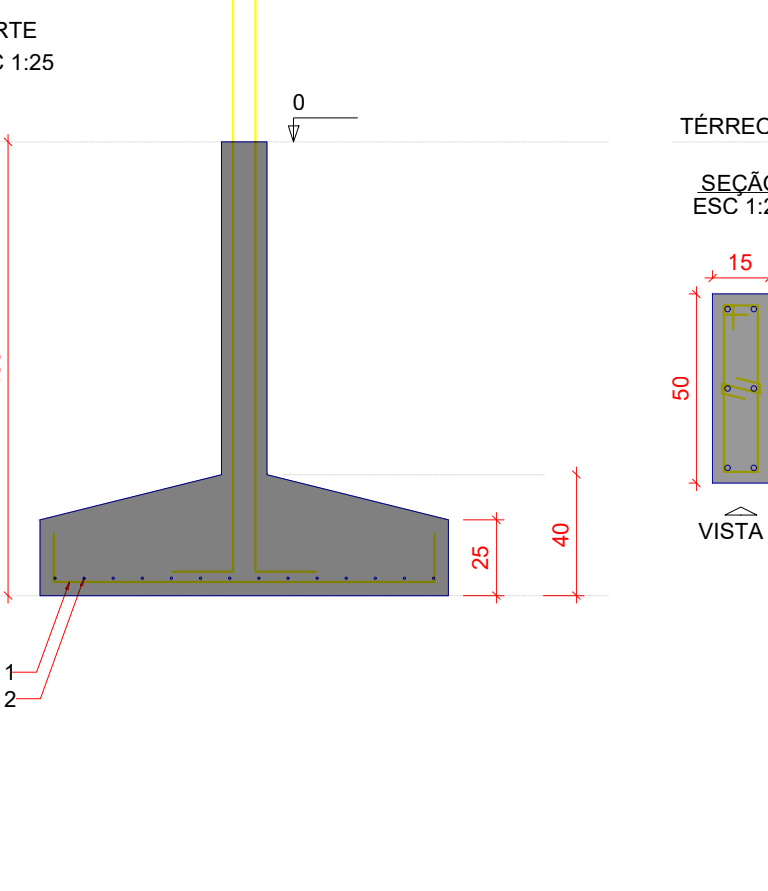
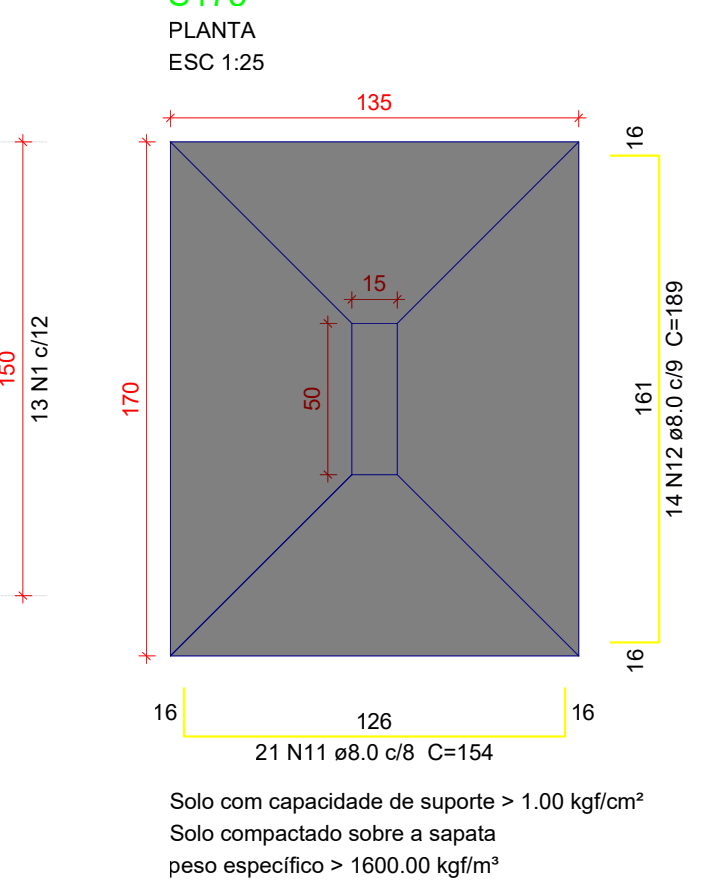
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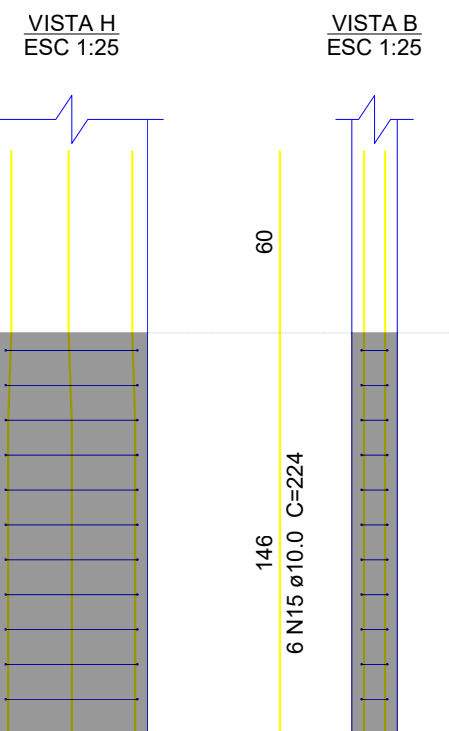
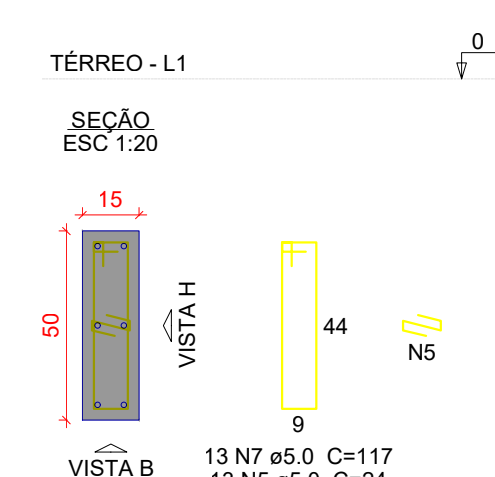
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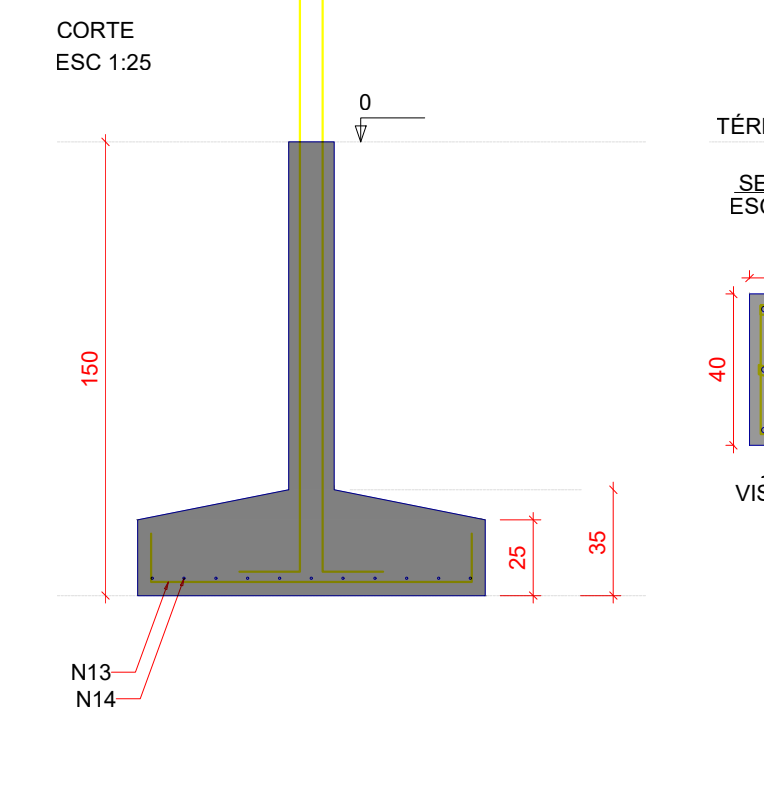
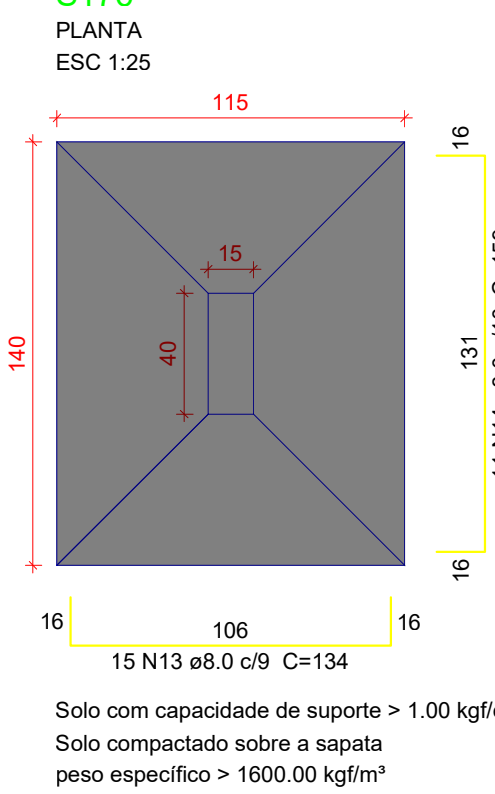
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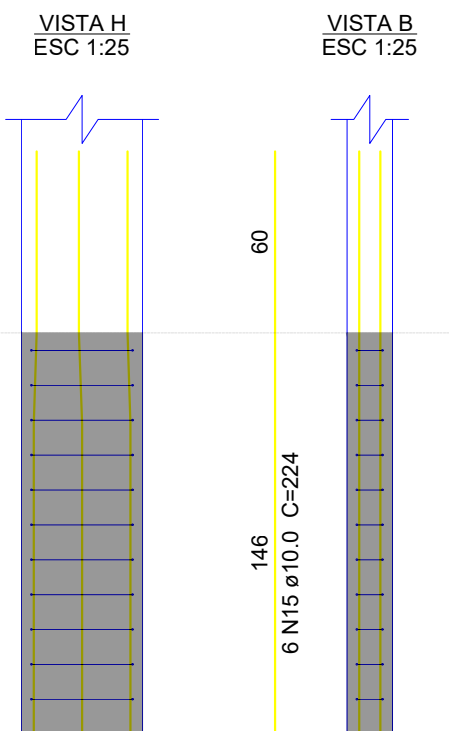
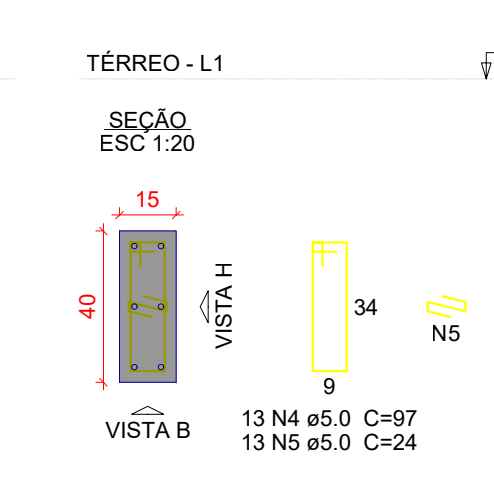
P175



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DETALHE ESQUEMÁTICO DAS ARMADURAS DAS SAPATAS EM 3D

OBS: SOMENTE EXECUTAR A FUNDAÇÃO DESTE EMPREENDIMENTO APÓS O ESTUDO DO SOLO, COM NO MÍNIMO O ENSAIO DE SONDAGEM TIPO SPT E A CONCLUSÃO DE UM ENGENHEIRO CALCULISTA DE FUNDAÇÕES AUTORIZANDO SUA EXECUÇÃO. VALE RESSALTAR QUE CADA SOLO É/OU REGIÃO EXISTE UM PERFIL GEOTÉCNICO DIFERENTE, O QUE OBRIGA QUE A FUNDAÇÃO SEJA RECALCULADA PARA EVITAR FUTURAS PATOLOGIAS E PROBLEMAS CONSTRUTIVOS, ALÉM DE UMA POSSÍVEL DIFERENÇA DE CUSTO SIGNIFICATIVA.

Características do Projeto	5 – OS VENTOS INCIDENTES NAS FACES X (90°) E Y (0°), RESPECTIVAMENTE, NÃO OCORREM SIMULTANEAMENTE.	LEGENDA DA PLANTA DE LOCAÇÃO
1 – COBRIMENTO DAS ARMADURAS – PILARES E VIGAS: 3 cm		A ORIENTAÇÃO DOS EIXOS DOS PILARES
2 – COBRIMENTO DAS ARMADURAS – LAJES E ESCADAS: 3 cm		1 ORIENTAÇÃO DOS EIXOS DOS PILARES
3 – COBRIMENTO DAS ARMADURAS – FUNDAÇÃO: 4.5 cm		
4 – PREVER LASTRO DE CONCRETO MAGRO (5 cm) SOB AS ESTRUTURAS EM CONCRETO.		
NOTAS 1 : DURABILIDADE	NOTAS 2 : NORMAS	NOTAS 3 : GERAIS
1 – CLASSE DE AGRESSIVIDADE AMBIENTAL: II	– NBR 06118 – 2023 – Projeto de Estruturas de Concreto armado	1 – Dimensões em Centímetros e Níveis em metros
2 – MÓDULO DE ELASTICIDADE > 35.42 GPa	– NBR 06120 – 2019 – Cargas para o Cálculo de Estruturas de edificações – Procedimento	2 – Conferir as disposições das armaduras antes do concretagem.
3 – FATOR A/C < 0.4	– NBR 06123 – 2023 – Forças Devidas ao Vento em Edificações	3 – A Responsabilidade pela fiscalização da obra é do Engº resp Técnico.
4 – AÇO CA 50A e CA 60B		4 – Aconselhamos moldagem de corpos de prova para cada combinação betoneira.
5 – CONCRETO CLASSE > 35 MPa	– NBR 8681 – 2003 – Ações e Segurança nas Estruturas	5 – Respeitar os prazos mínimos para retirada de formas e escoramentos.
6 – CONSUMO DE CIMENTO > 350 Kg/m3	– NBR 6122 – 2022 – Projeto e execução de Fundações	6 – Evitar romper concreto após endurecido, com moirões e talhadeira.
		7 – Toda e qualquer alteração no respectivo projeto, o Calculista deverá ser consultado e o mesmo deverá emitir seu parecer por escrito.

MUNICÍPIO DE ITACOATIARA		MUNICÍPIO DE ITACOATIARA	
DISCIPLINA: PROJETOS		DISCIPLINA: PROJETOS	
PROFESSOR: MS. POLICLÍNICA, EST-08		PROFESSOR: MS. POLICLÍNICA, EST-08	
ALUNO: KAYO HENRIQUE MOREIRA		ALUNO: KAYO HENRIQUE MOREIRA	
TURMA: 19974/D		TURMA: 19974/D	